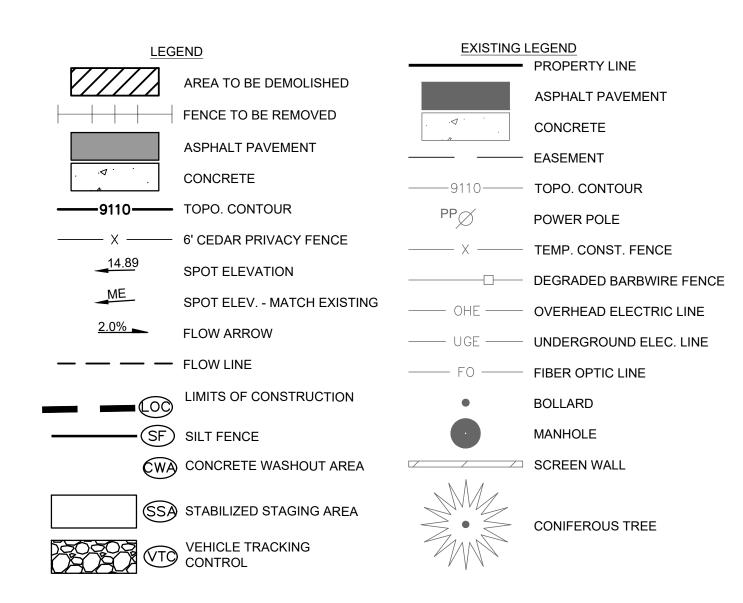
CENTURYLINK DILLON

166 LAKE DILLON DRIVE DILLON, CO



CIVIL DRAWING INDEX:

CIVIL COVER SHEET C-101 CIVIL SPECIFICATIONS C-200 DEMOLITION PLAN C-300 SITE PLAN C-400 **GRADING PLAN EROSION CONTROL PLAN** C-500 SITE DETAILS **EROSION CONTROL DETAILS**

GRADING AND EROSION CONTROL NOTES

1. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS AND IMPLEMENTING AND MAINTAINING EROSION AND SEDIMENT CONTROL MEASURES AT ALL TIMES DURING CONSTRUCTION TO PREVENT DAMAGING FLOWS ON THE SITE AND IN THE WATERSHED BELOW THE SITE. CONTROL SYSTEMS SHALL BE INSTALLED PRIOR TO START OF CONSTRUCTION. CONTROL SYSTEMS SHALL INCLUDE GRAVEL BAG ROCK SOCKS, SILT FENCES AND INLET PROTECTION DEVICES.

- 2. THE CONTRACTOR SHALL PROVIDE ANY ADDITIONAL DUST ABATEMENT AND EROSION CONTROL MEASURES AS SPECIFIED BY THE SWMP.
- 3. THE CONTRACTOR SHALL PREVENT SEDIMENT, DEBRIS AND ALL OTHER POLLUTANTS FROM LEAVING THE SITE DURING ALL DEMOLITION, EXCAVATION, TRENCHING, GRADING OR OTHER CONSTRUCTION OPERATIONS THAT ARE PART OF THIS PROJECT. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR REMEDIATION OF ANY ADVERSE IMPACTS TO ADJACENT WATERWAYS, WETLANDS, ETC., RESULTING FROM WORK DONE AS PART OF THIS PROJECT.
- 4. THE CONTRACTOR SHALL INSURE THAT ALL LOADS OF CUT AND FILL MATERIAL IMPORTED TO OR EXPORTED FROM THE SITE SHALL BE PROPERLY COVERED TO PREVENT LOSS OF THE MATERIAL DURING TRANSPORT ON PUBLIC ROADS.
- 5. APPROVED EROSION AND SEDIMENT CONTROL "BEST MANAGEMENT PRACTICES" (BMP'S) SHALL BE MAINTAINED AND KEPT IN GOOD REPAIR FOR THE DURATION OF THE PROJECT. AT A MINIMUM, THE CONTRACTOR OR HIS SWMP ADMINISTRATOR SHALL INSPECT ALL BMPS WEEKLY AND AFTER SIGNIFICANT PRECIPITATION EVENTS. ALL NECESSARY MAINTENANCE AND REPAIRS SHALL BE COMPLETED WITHIN 48 HOURS OF DISCOVERY. ACCUMULATED SEDIMENT AND DEBRIS SHALL BE REMOVED FROM A BMP WHEN THE SEDIMENT LEVEL REACHES ONE HALF THE HEIGHT OF THE BMP OR, AT ANY TIME THAT SEDIMENT OF DEBRIS ADVERSELY IMPACT THE FUNCTIONING OF THE BMP.
- 6. THE CONTRACTOR SHALL PROTECT ALL STORM SEWER FACILITIES ADJACENT TO ANY LOCATION WHERE PAVEMENT SAW CUTTING OPERATIONS ARE TO TAKE PLACE. THE CONTRACTOR SHALL REMOVE AND PROPERLY DISPOSE OF ALL WASTE PRODUCTS GENERATED BY THE CUTTING OPERATIONS ON A DAILY BASIS. THE DISCHARGE OF ANY WATER CONTAMINATED BY WASTE PRODUCTS FROM CUTTING OPERATIONS TO THE DRAINAGE SYSTEM IS PROHIBITED.
- 7. PAVED SURFACES ADJACENT TO THE SITE SHALL BE SWEPT IN A TIMELY MANNER WHEN SEDIMENT AND OTHER MATERIALS ARE TRACKED OR DISCHARGED ONTO THEM. EITHER SWEEPING BY HAND OR USE OF STREET SWEEPERS IS ACCEPTABLE. STREET SWEEPERS USING WATER WHILE SWEEPING IS PREFERRED TO MINIMIZE DUST. FLUSHING OFF PAVED SURFACES WITH WATER IS PROHIBITED.
- 8. EROSION/SEDIMENT CONTROL DEVICES PER SWMP PLANS SHALL BE PLACED AS CONSTRUCTION SEQUENCING AND ACCESS DICTATES.

CIVIL GENERAL NOTES:

- 1. THE CONTRACTOR IS RESPONSIBLE FOR ESTABLISHING LINE AND GRADE, IN CONFORMANCE WITH THE INFORMATION SHOWN ON THE PLANS, FOR ALL FACILITIES TO BE CONSTRUCTED AS PART OF THE SCOPE OF THIS PROJECT. THIS INCLUDES RESTORING TO THEIR ORIGINAL LINE AND GRADE ALL SITE IMPROVEMENTS THAT ARE REMOVED AND REPLACED AS PART OF THIS PROJECT. THE ENGINEER OR OWNER MAY, FROM TIME TO TIME, CHECK THE LINE AND GRADE OF COMPLETED SECTIONS OF THE WORK OR SECTION OF THE WORK IN PROGRESS. THIS, HOWEVER, IN NO WAY RELIEVES THE CONTRACTOR OF HIS RESPONSIBILITY IN ESTABLISHING THE LINE AND GRADE OF THE WORK IN CONFORMANCE WITH THE PLANS.
- 2. EXISTING INFORMATION SHOWN ON THE DRAWINGS HAS BEEN TAKEN FROM OWNER FURNISHED DRAWINGS AND LIMITED FIELD OBSERVATIONS. ANDERSON & HASTINGS CONSULTANTS, INC. IS NOT RESPONSIBLE FOR THE ACCURACY OF ANY OWNER-PROVIDED INFORMATION OR THE ADEQUACY, SAFETY AND CONFORMANCE TO CURRENT PREVAILING CODES OF ANY WORK SHOWN AS EXISTING ON THESE DRAWINGS.
- 3. EXISTING UTILITIES SHOWN ARE BASED ON THE BEST INFORMATION AVAILABLE TO THE ENGINEER. PRIOR TO EXCAVATING, THE CONTRACTOR SHALL FIELD LOCATE (INCLUDING DEPTHS) BY POTHOLING ALL EXISTING UTILITIES WHICH MAY CONFLICT WITH THE PROPOSED CONSTRUCTION. FIBER OPTIC AND BURIED PHONE LINES SHALL BE EXPOSED BY AIR POTHOLING. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE REPAIR OF DAMAGE TO UTILITIES SHOWN HEREON AT HIS OWN EXPENSE.
- 4. WHEN THE CONTRACTOR DISCOVERS ANY CONFLICT BETWEEN THE DESIGN LOCATION OF WORK UNDER THIS CONTRACT AND AN EXISTING UTILITY HE SHALL NOTIFY THE OWNER AND THE ENGINEER IMMEDIATELY.
- 5. IF THE CONTRACTOR ENCOUNTERS ANY UNDOCUMENTED LINES, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER AND NOTIFY THE ENGINEER SO THAT ANY CONFLICTS CAN BE MITIGATED.
- 6. ALL EXISTING UTILITIES SHALL BE PROTECTED FROM DAMAGE BY THE CONTRACTOR. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE REPAIR OF DAMAGE TO UTILITIES SHOWN HEREIN AT HIS OWN EXPENSE.
- 7. EXISTING FENCES, SITE IMPROVEMENTS (INCLUDING LANDSCAPING & IRRIGATION) AND STRUCTURES SHALL BE PROTECTED FROM CONSTRUCTION RELATED DAMAGE. CONTRACTOR SHALL CORRECT OR RESTORE SUCH DAMAGE AT CONTRACTOR'S
- 8. CONTRACTOR SHALL MINIMIZE CONSTRUCTION RELATED MUD AND DEBRIS ON SURFACES ADJACENT TO THE PROJECT SITE -LANDSCAPE, STREETS, DRIVES AND SIDEWALKS.
- 9. EQUIPMENT AND MATERIALS NOT INCORPORATED IN PROJECT SCOPE SHALL NOT BE STORED OR STOCKED ON SITE.
- 10. THE CONTRACTOR SHALL ADHERE TO THE NOISE ORDINANCE OF THE CONTROLLING MUNICIPAL AUTHORITY
- 11. THE CONTRACTOR SHALL ADHERE TO THE OWNER'S REQUIREMENTS FOR CONSTRUCTION PARKING AND SHALL REQUIRE THAT THEIR EMPLOYEES AND ALL SUB-TRADE EMPLOYEES PARK ONLY IN DESIGNATED AREA(S). THE OWNER'S CONTACT WILL ADVISE THE CONTRACTOR ON THE SPECIFICS OF THESE REQUIREMENTS.
- 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE RIM OF ALL EXISTING AND PROPOSED MANHOLES, CLEANOUTS AND INLETS TO FINISHED GRADE.
- 13. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED CITY, COUNTY AND/OR STATE PERMITS PRIOR TO CONSTRUCTION.
- 14. CONTRACTOR IS RESPONSIBLE FOR CONTROL OF EROSION OF ALL AREAS OF CONSTRUCTION. THE STORM WATER MANAGEMENT PLANS INCLUDED IN THE DRAWING SET ARE CONCEPTUAL ONLY AND DO NOT RELIEVE THE CONTRACTOR OF THE FULL RESPONSIBILITY FOR EROSION AND SEDIMENT CONTROL IN ALL AREAS OF CONSTRUCTION.
- 15. TRENCHES AND OPEN EXCAVATIONS SHALL BE APPROPRIATELY MARKED AND PROTECTED BY THE CONTRACTOR. NO TRENCHES OR EXCAVATIONS SHALL REMAIN UNPROTECTED OVERNIGHT. CONTRACTOR SHALL PROVIDE PROTECTION SUFFICIENT TO MAINTAIN PUBLIC SAFETY, IN ACCORDANCE WITH THE CONTROLLING MUNICIPAL AUTHORITY AND OSHA REGULATIONS AS WELL AS ANY REQUIREMENTS OF THE OWNER.
- 16. THE METHODS OF CONSTRUCTION SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND SUBCONTRACTORS. THE CONTRACTORS SHALL TAKE ALL REASONABLE PRECAUTIONS TO PROTECT WORK IN PROGRESS. HIS PERSONNEL AND VISITORS TO THE SITE FROM SOURCES OF INJURY. SUCH PRECAUTIONS SHALL INCLUDE BUT ARE NOT LIMITED TO: ERECTION OF BARRIERS, SHORING AND/OR BRACING OF EXCAVATIONS AND PROTECTION OF WORK IN PLACE FROM INTRUSION BY TRESPASS
- 17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER DISPOSAL OF ALL TRASH, DEBRIS AND EXCAVATED MATERIAL GENERATED AS A RESULT OF HIS/HER WORK.
- 18. SHOP DRAWINGS, PRODUCT DATA, AND TECHNICAL INFORMATION FOR ALL PRODUCTS THAT ARE TO BE INSTALLED AS PART OF THIS PROJECT SHALL BE ELECTRONICALLY SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR APPROVAL. SUBMITTALS SHALL BE E-MAILED TO THE ENGINEER WITH COPIES E-MAILED TO THE OWNER'S PROJECT MANAGER.
- 19. GEOTECHNICAL ENGINEERING AND TESTING SHALL BE PERFORMED BY THE OWNER'S GEOTECHNICAL ENGINEER AND PAID FOR BY THE OWNER. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE WITH THE GEOTECHNICAL ENGINEER TO SCHEDULE ALL TESTING AND OBSERVATION REQUIRED BY THE PROJECT DOCUMENTS.
- 20. THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXPORTING AND DISPOSAL OF EXCESS EXCAVATED MATERIAL OR THE IMPORT OF FILL MATERIAL AS REQUIRED BY THE GRADING PLANS.
- 21. THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS AT AND ADJACENT TO THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- 22. OWNER/ENGINEER CONSTRUCTION REVIEW OF THE CONTRACTORS PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON OR NEAR THE CONSTRUCTION SITE.
- 23. ALL CONSTRUCTION ACTIVITIES EXCEEDING 1 ACRE OF DISTURBANCE MUST COMPLY WITH THE STATE OF COLORADO PERMITTING PROCESS FOR "STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY. CONTACT THE COLORADO DEPARTMENT OF HEALTH, WATER QUALITY DIVISION FOR INFORMATION.
- 24. THE CONTRACTORS SHALL FULLY FAMILIARIZE HIM/HER SELF WITH THE REQUIREMENTS AS REPRESENTED IN THE DRAWINGS AND WITH THE CONDITIONS AT THE SITE. NO ADDITIONAL COSTS TO THE OWNER SHALL BE ACCEPTED FOR ADDITIONAL WORK FOR FORESEEABLE OR EXISTING CONDITIONS.
- 25. SITE BENCHMARK: NORTH PROPERTY CORNER, #3 REBAR WITHOUT CAP. ELEVATION = 9116.66' (NAVD 88 DATUM)
- 26. EXISTING TOPOGRAPHY SHOWN WAS TAKEN FROM A FIELD SURVEY DATED 3/16/2020 BY ANDERSON & HASTINGS CONSULTANTS, INC. BENCHMARK ELEVATIONS AND DATUM ARE AS NOTED.
- 27. ALL EARTHWORK AND GRADING TO BE IN CONFORMANCE WITH THE GEOTECHNICAL STUDY REPORT PREPARED BY ______ FOR SITE DATED PROJECT#



All information appearing herein shall not be duplicated, discharged o

Gudmundur Jonsson

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CIVIL ENGINEER Anderson & Hastings Consultants, Inc. 12596 W Bayaud Ave, Suite 200 Lakewood, CO 80228

Permit Drawings	3/24/20
REV. Permit Drawings	5/28/20
Issues/Revisions:	Date:

DILLON PARKING STRUCTURE 166 LAKE DILLON DRIVE DILLON, COLORADO

Drawn by: CDB

Checked by: JVH

CIVIL COVER SHEET

CIVIL SPECIFICATIONS:

SECTION 02 41 00 - SELECTIVE DEMOLITION

- 1.1 FIELD CONDITIONS
- A. Owner will occupy building immediately adjacent to selective demolition area. Conduct selective demolition so Owner's operations will not be disrupted.
- B. Notify Architect of discrepancies between existing conditions and Drawings before proceeding with selective demolition.
- C. Hazardous Materials: It is not expected that hazardous materials will be encountered in the Work.
- 1. Hazardous materials will be removed by Owner before start of the Work. 2. If suspected hazardous materials are encountered, do not disturb; immediately notify Architect and Owner. Hazardous materials will be removed by Owner under a separate contract.
- D. Utility Service: Maintain existing utilities indicated to remain in service and protect them against damage during selective demolition operations.
- 1.2 PEFORMANCE REQUIREMENTS A. Regulatory Requirements: Comply with governing EPA notification regulations before beginning selective demolition. Comply with hauling and disposal regulations of authorities having jurisdiction.
- 1.3 EXAMINATION
- A. Verify that utilities have been disconnected and capped before starting selective demolition operations. B. Survey existing conditions and correlate with requirements indicated to determine extent of selective demolition required.
- C. When unanticipated mechanical, electrical, or structural elements that conflict with intended function or design are encountered, investigate and measure the nature and extent of conflict. Promptly submit a written report to Architect.
- 1.4 PREPARATION A. Site Access and Temporary Controls: Conduct selective demolition and debris-removal operations to ensure minimum interference with roads, streets, walks, walkways, and other adjacent occupied and used facilities.
- B. Temporary Facilities: Provide temporary barricades and other protection required to prevent injury to people and damage to adjacent buildings and facilities to remain.
- 1.5 SELECTIVE DEMOLITION, GENERAL A. General: Demolish and remove existing construction only to the extent required by new construction and as indicated. Use methods required to complete the Work within limitations of governing
- regulations and dispose removed materials off of Air Force Property. 1.6 CLEANING
- A. Clean adjacent structures and improvements of dust, dirt, and debris caused by selective demolition operations. Return adjacent areas to condition existing before selective demolition operations

31 00 00 - EARTH MOVING

- 1.1 DEFINITIONS
- A. Backfill: Soil material used to fill an excavation.
- 1. Initial Backfill: Backfill placed beside and over pipe in a trench, including haunches to support sides of pipe. 2. Final Backfill: Backfill placed over initial backfill to fill a trench.
- B. Bedding Course: Aggregate layer placed over the excavated subgrade in a trench before laying pipe.
- A. Satisfactory Soils: Soil Classification Groups GW, GP, GM, SW, SP, and SM according to ASTM D 2487, or a combination of these groups; free of rock or gravel larger than 3 inches in any dimension, debris, waste, frozen materials, vegetation, and other deleterious matter.
- 1. Liquid Limit: 30.
- 2. Plasticity Index: 15.
- B. Unsatisfactory Soils: Soil Classification Groups GC, SC, CL, ML, OL, CH, MH, OH, and PT according to ASTM D 2487, or a combination of these groups. 1. Unsatisfactory soils also include satisfactory soils not maintained within 2 percent of optimum moisture content at time of compaction.
- C. Bedding Course: Naturally or artificially graded mixture of natural or crushed stone, and natural or crushed sand; ASTM D 2940; except with 100 percent passing a 1-inch sieve and not more than 8 percent passing a No. 200 sieve.
- 1.3 ACCESSORIES A. Detectable Warning Tape: Acid- and alkali-resistant, polyethylene film warning tape manufactured for marking and identifying underground utilities, a minimum of 4 inches wide and 4 mils thick,
- continuously inscribed with a description of the utility, with metallic core encased in a protective jacket for corrosion protection, detectable by metal detector when tape is buried up to 30 inches deep; colored to comply with local practice or requirements of authorities having jurisdiction. 1.4 PREPARATION
- A. Protect and maintain erosion and sedimentation controls during earth moving operations.
- A. Unclassified Excavation: Excavate to subgrade elevations regardless of the character of surface and subsurface conditions encountered. Unclassified excavated materials may include rock, soil materials, and obstructions. No changes in the Contract Sum or the Contract Time will be authorized for rock excavation or removal of obstructions.
- 1. If excavated materials intended for fill and backfill include unsatisfactory soil materials and rock, replace with satisfactory soil materials. 1.6 FXCAVATION
- A. Excavate to indicated elevations and dimensions. If applicable, extend excavations a sufficient distance from structures for placing and removing concrete formwork, for installing services and other construction, and for inspections.
- 1. Excavations for Footings and Foundations: Do not disturb bottom of excavation. Excavate by hand to final grade just before placing concrete reinforcement. Trim bottoms to required lines and grades to leave solid base to receive other work.
- A. Proof-roll subgrade below the building slabs and pavements with a pneumatic-tired dump truck to identify soft pockets and areas of excess yielding. Do not proof-roll wet or saturated subgrades.
- B. Reconstruct subgrades damaged by freezing temperatures, frost, rain, accumulated water, or construction activities, as directed by Architect, without additional compensation.
- A. Fill unauthorized excavation under foundations or wall footings by extending bottom elevation of concrete foundation or footing to excavation bottom, without altering top elevation. Lean concrete fill, with 28-day compressive strength of 2500 psi, may be used when approved by Architect. 1. Fill unauthorized excavations under other construction, pipe, or conduit as directed by Architect.
- 1.9 STORAGE OF SOIL MATERIALS A. Stockpile borrow soil materials and excavated satisfactory soil materials without intermixing. Place, grade, and shape stockpiles to drain surface water. Cover to prevent windblown dust and install
- erosion control measures.
- 1. Stockpile soil materials away from edge of excavations. Do not store within drip line of remaining trees. 1.10 SOIL MOISTURE CONTROL
- A. Uniformly moisten or aerate subgrade and each subsequent fill or backfill soil layer before compaction to within 2 percent of optimum moisture content. 1. Do not place backfill or fill soil material on surfaces that are muddy, frozen, or contain frost or ice.
- 2. Remove and replace, or scarify and air dry, otherwise satisfactory soil material that exceeds optimum moisture content by 2 percent and is too wet to compact to specified dry unit weight.
- 1.11 COMPACTION OF SOIL BACKFILLS AND FILLS A. Place backfill and fill satisfactory soil materials in layers not more than 8 inches in loose depth for material compacted by heavy compaction equipment, and not more than 4 inches in loose
- depth for material compacted by hand-operated tampers.
- B. Place backfill and fill satisfactory soil materials evenly on all sides of structures to required elevations, and uniformly along the full length of each structure. C. Compact soil materials to not less than the following percentages of maximum dry unit weight according to ASTM D 698:
- 1. Under structures, building slabs, steps, and pavements, scarify and recompact top 12 inches of existing subgrade and each layer of backfill or fill soil material at 95 percent. 2. For utility trenches, compact each layer of initial and final backfill soil material at 95% percent.
- 1.12 GRADING A. General: Uniformly grade areas to a smooth surface, free of irregular surface changes. Comply with compaction requirements and grade to cross sections, lines, and elevations indicated.
- B. Site Rough Grading: Slope grades to direct water away from buildings and to prevent ponding. Finish subgrades to required elevations within the following tolerances:
- 5. Pavements: Plus or minus 1/2 inch. C. Grading inside Building Lines: Finish subgrade to a tolerance of 1/2 inch when tested with a 10-foot straightedge.
- 1.13 FIELD QUALITY CONTROL
- A. Testing Agency: Owner will engage a qualified geotechnical engineering testing agency to perform tests and inspections.
- B. Allow testing agency to inspect and test subgrades and each fill or backfill layer. Proceed with subsequent earth moving only after test results for previously completed work comply with
- C. Footing Subgrade: At footing subgrades, at least one test of each soil stratum will be performed to verify design bearing capacities. Subsequent verification and approval of other footing subgrades may be based on a visual comparison of subgrade with tested subgrade when approved by Architect.
- D. When testing agency reports that subgrades, fills, or backfills have not achieved degree of compaction specified, scarify and moisten or aerate, or remove and replace soil materials to depth required; recompact and retest until specified compaction is obtained.
- 1.14 DISPOSAL OF SURPLUS AND WASTE MATERIALS
- A. Remove surplus satisfactory soil and waste materials, including unsatisfactory soil, trash, and dispose off site.

SECTION 31 12 17 - ASPHALT PAVING

- PART 1 GENERAL
- 1.1 ACTION SUBMITTALS
- A. Product Data: For each type of product indicated. Include technical data and tested physical and performance properties.
- 1. Job-Mix Designs: For each job mix proposed for the Work.
- 2. Material certificates for each paving material, from manufacturer. 1.2 QUALITY ASSURANCE
- A. Regulatory Requirements: Comply with materials, workmanship, and other applicable requirements of CDOT and the City of Dillon for asphalt paving work.
- 1.3 AGGREGATES
- A. Coarse Aggregate: ASTM D 692, sound; angular crushed stone, crushed gravel, or cured, crushed blast-furnace slag. B. Fine Aggregate: ASTM D 1073, sharp-edged natural sand or sand prepared from stone, gravel, cured blast-furnace slag, or combinations thereof.
- C. Mineral Filler: ASTM D 242, rock or slag dust, hydraulic cement, or other inert material. 1.4 ASPHALT MATERIALS
- A. Asphalt Binder: AASHTO M 320 or AASHTO MP 1a. PG 64-22. B. Tack Coat: AASHTO M 140 emulsified asphalt, or AASHTO M 208 cationic emulsified asphalt, slow setting, diluted in water, of suitable grade and consistency for application.
- A. Hot-Mix Asphalt: Dense, hot-laid, hot-mix asphalt plant mixes approved by authorities having jurisdiction; designed according to procedures in AASHTO T-312 or Colorado Procedure CP-L 5115 for
- th eSuperpave Method of Mixture Design.:
- 1. Base Course: CDOT Grade S. 2. Surface Course: CDOT Grade SX.
- 1.6 EXAMINATION
- A. Proof-roll subgrade below pavements with heavy pneumatic-tired equipment to identify soft pockets and areas of excess yielding. Do not proof-roll wet or saturated subgrades.
- B. Proceed with paying only after unsatisfactory conditions have been corrected.
- 1.7 PATCHING A. Hot-Mix Asphalt Pavement: Saw cut perimeter of patch and excavate existing pavement section to sound base. Excavate rectangular or trapezoidal patches, extending 12 inches into adjacent
- sound pavement, unless otherwise indicated. Cut excavation faces vertically. Remove excavated material. Recompact existing unbound-aggregate base course to form new subgrade. B. Tack Coat: Apply uniformly to vertical surfaces abutting or projecting into new, hot-mix asphalt paving at a rate of 0.05 to 0.15 gal./sq. yd..
- 1. Allow tack coat to cure undisturbed before applying hot-mix asphalt paving. 2. Avoid smearing or staining adjoining surfaces, appurtenances, and surroundings. Remove spillages and clean affected surfaces.
- C. Patching: Fill excavated pavements with hot-mix asphalt base mix for full thickness of patch and, while still hot, compact flush with adjacent surface.
- 1.8 SURFACE PREPARATION
- A. General: Immediately before placing asphalt materials, remove loose and deleterious material from substrate surfaces. Ensure that prepared subgrade is ready to receive paving.
- B. Tack Coat: Apply uniformly to surfaces of existing pavement at a rate of 0.05 to 0.15 gal./sq. yd..
- 1. Allow tack coat to cure undisturbed before applying hot-mix asphalt paving.
- 2. Avoid smearing or staining adjoining surfaces, appurtenances, and surroundings. Remove spillages and clean affected surfaces.
- 1.9 HOT-MIX ASPHALT PLACING A. Machine place hot-mix asphalt on prepared surface, spread uniformly, and strike off. Place asphalt mix by hand to areas inaccessible to equipment in a manner that prevents segregation of mix. Place each course to required grade, cross section, and thickness when compacted.
- 3. Spread mix at minimum temperature of 250 deg F.
- 4. Regulate paver machine speed to obtain smooth, continuous surface free of pulls and tears in asphalt-paving mat. B. Place paying in consecutive strips not less than 10 feet wide unless infill edge strips of a lesser width are required.
- C. Promptly correct surface irregularities in paving course behind paver. Use suitable hand tools to remove excess material forming high spots. Fill depressions with hot-mix asphalt to prevent
- segregation of mix; use suitable hand tools to smooth surface. 1.10 JOINTS
- A. Construct joints to ensure a continuous bond between adjoining paving sections. Construct joints free of depressions, with same texture and smoothness as other sections of hot-mix asphalt
- 1. Clean contact surfaces and apply tack coat to joints.
- 2. Offset longitudinal joints, in successive courses, a minimum of 6 inches.
- 3. Offset transverse joints, in successive courses, a minimum of 24 inches.
- 4. Construct transverse joints at each point where paver ends a day's work and resumes work at a subsequent time. Construct these joints using either "bulkhead" or "papered" method according to AI MS-22, for both "Ending a Lane" and "Resumption of Paving Operations." 1.11 COMPACTION
- A. General: Begin compaction as soon as placed hot-mix paving will bear roller weight without excessive displacement. Compact hot-mix paving with hot, hand tampers or with vibratory-plate compactors in areas inaccessible to rollers.
- 1. Complete compaction before mix temperature cools to 185 dea F. B. Breakdown Rolling: Complete breakdown or initial rolling immediately after rolling joints and outside edge. Examine surface immediately after breakdown rolling for indicated crown, grade, and smoothness. Correct laydown and rolling operations to comply with requirements.
- C. Intermediate Rolling: Begin intermediate rolling immediately after breakdown rolling while hot-mix asphalt is still hot enough to achieve specified density. Continue rolling until hot-mix asphalt course has been uniformly compacted to the following density:
- 1. Average Density: 92 percent of reference maximum theoretical density according to ASTM D 2041, but not less than 90 percent nor greater than 96 percent. D. Finish Rolling: Finish roll paved surfaces to remove roller marks while hot-mix asphalt is still warm.
- E. Edge Shaping: While surface is being compacted and finished, trim edges of pavement to proper alignment. Bevel edges while asphalt is still hot; compact thoroughly. F. Protection: After final rolling, do not permit vehicular traffic on pavement until it has cooled and hardened.
- G. Erect barricades to protect paving from traffic until mixture has cooled enough not to become marked.
- 1.12 INSTALLATION TOLERANCES A. Pavement Thickness: Compact each course to produce the thickness indicated within the following tolerances:
- 1. Base Course: Plus or minus 1/2 inch.
- 2. Surface Course: Plus 1/4 inch. no minus. B. Pavement Surface Smoothness: Compact each course to produce a surface smoothness within the following tolerances as determined by using a 10-foot straightedge applied transversely or
- longitudinally to payed areas: 1. Base Course: 1/4 inch.
- 2. Surface Course: 1/8 inch.
- 1.13 FIELD QUALITY CONTROL A. Testing Agency: Owner will engage a qualified testing agency to perform tests and inspections.
- B. Replace and compact hot-mix asphalt where core tests were taken. C. Remove and replace or install additional hot-mix asphalt where test results or measurements indicate that it does not comply with specified requirements.
- A. Except for material indicated to be recycled, remove excavated materials from Project site and legally dispose of them in off-site.

SECTION 32 13 13 - CONCRETE PAVING

- PART 1 GENERAL 1.1 ACTION SUBMITTALS
- A. Submittals:
- 1. Design Mixtures: For each concrete paving mixture. Include alternate design mixtures when characteristics of materials, Project conditions, weather, test results, or other circumstances warrant adjustments.
- 1.2 STEEL REINFORCEMENT A. Reinforcing Bars: ASTM A 615/A 615M, Grade 60; deformed.
- B. Dowel Bars: ASTM A 615/A 615M, Grade 60 plain-steel bars. Cut bars true to length with ends square and free of burrs. 1.3 CONCRETE MATERIALS
- A. Conforming to CDOT Class B. 1.4 FIBER REINFORCEMENT
- A. Synthetic Fiber: Monofilament or fibrillated polypropylene fibers engineered and designed for use in concrete paving, complying with ASTM C 1116/C 1116M, Type III, 1/2 to 1-1/2 inches long.
- 1.5 RELATED MATERIALS A. Joint Fillers: ASTM D 1751, asphalt-saturated cellulosic fiber or ASTM D 1752, cork or self-expanding cork in preformed strips.
- 1.6 CONCRETE MIXTURES A. Prepare design mixtures, proportioned according to ACI 301, with the following properties:
- 1. Compressive Strength (28 Days): 4500 psi. 2. Maximum Water-Cementitious Materials Ratio at Point of Placement: 0.45.
- 3. Slump Limit: 4 inches, plus or minus 1 inch.
- 4. Air Content: 6 percent plus or minus 1.5 percent. 5. Cement: CDOT Class P
- B. Chemical Admixtures: Use admixtures according to manufacturer's written instructions.
- C. Synthetic Fiber: Uniformly disperse in concrete mixture at manufacturer's recommended rate, but not less than 1.5 lb/cu. yd.. 1.7 EDGE FORMS AND SCREED CONSTRUCTION
- A. Set, brace, and secure edge forms, bulkheads, and intermediate screed guides to required lines, grades, and elevations. Install forms to allow continuous progress of work and so forms can remain in place at least 24 hours after concrete placement.
- 1.8 STEEL REINFORCEMENT A. General: Comply with CRSI's "Manual of Standard Practice" for fabricating, placing, and supporting reinforcement.
- 1.9 JOINTS
- A. General: Form construction, isolation, and contraction joints and tool edges true to line, with faces perpendicular to surface plane of concrete. Construct transverse joints at right angles to centerline unless otherwise indicated.
- B. Construction Joints: Set construction joints at side and end terminations of paving and at locations where paving operations are stopped for more than one-half hour unless paving terminates at isolation joints. C. Isolation Joints: Form isolation joints of preformed joint-filler strips abutting concrete curbs, catch basins, manholes, inlets, structures, other fixed objects, and where indicated.
- D. Contraction Joints: Form weakened-plane contraction joints, sectioning concrete into areas as indicated. Construct contraction joints for a depth equal to at least one-fourth of the concrete thickness at 6 foot
- E. Edging: After initial floating, tool edges of paving, gutters, curbs, and joints in concrete with an edging tool to a 1/4-inch radius. Repeat tooling of edges after applying surface finishes. Eliminate edging-tool marks
- on concrete surfaces. F. Seal joints in a accordance with CDOT specifications.
- 1.10 CONCRETE PLACEMENT A. Comply with ACI 301 requirements for measuring, mixing, transporting, placing, and consolidating concrete.
- 1.11 FLOAT FINISHING
- A. General: Do not add water to concrete surfaces during finishing operations.
- B. Float Finish: Begin the second floating operation when bleed-water sheen has disappeared and concrete surface has stiffened sufficiently to permit operations. Float surface with power-driven floats or by hand floating if area is small or inaccessible to power units. Finish surfaces to true planes. Cut down high spots and fill low spots. Refloat surface immediately to uniform granular texture. 1. Medium-to-Fine-Textured Broom Finish: Draw a soft-bristle broom across float-finished concrete surface perpendicular to line of traffic to provide a uniform, fine-line texture.
- 1.12 CONCRETE PROTECTION AND CURING
- A. General: Protect freshly placed concrete from premature drying and excessive cold or hot temperatures. B. Comply with ACI 306.1 for cold-weather protection.
- 1.13 PAVING TOLERANCES A. Comply with tolerances in ACI 117 and as follows.
- 1. Elevation: 3/4 inch. 2. Thickness: Plus 3/8 inch, minus 1/4 inch.
- 3. Surface: Gap below 10-foot- long, unleveled straightedge not to exceed 1/2 inch. 4. Joint Spacing: 3 inches.
- 5. Contraction Joint Depth: Plus 1/4 inch, no minus.
- 6. Joint Width: Plus 1/8 inch, no minus.

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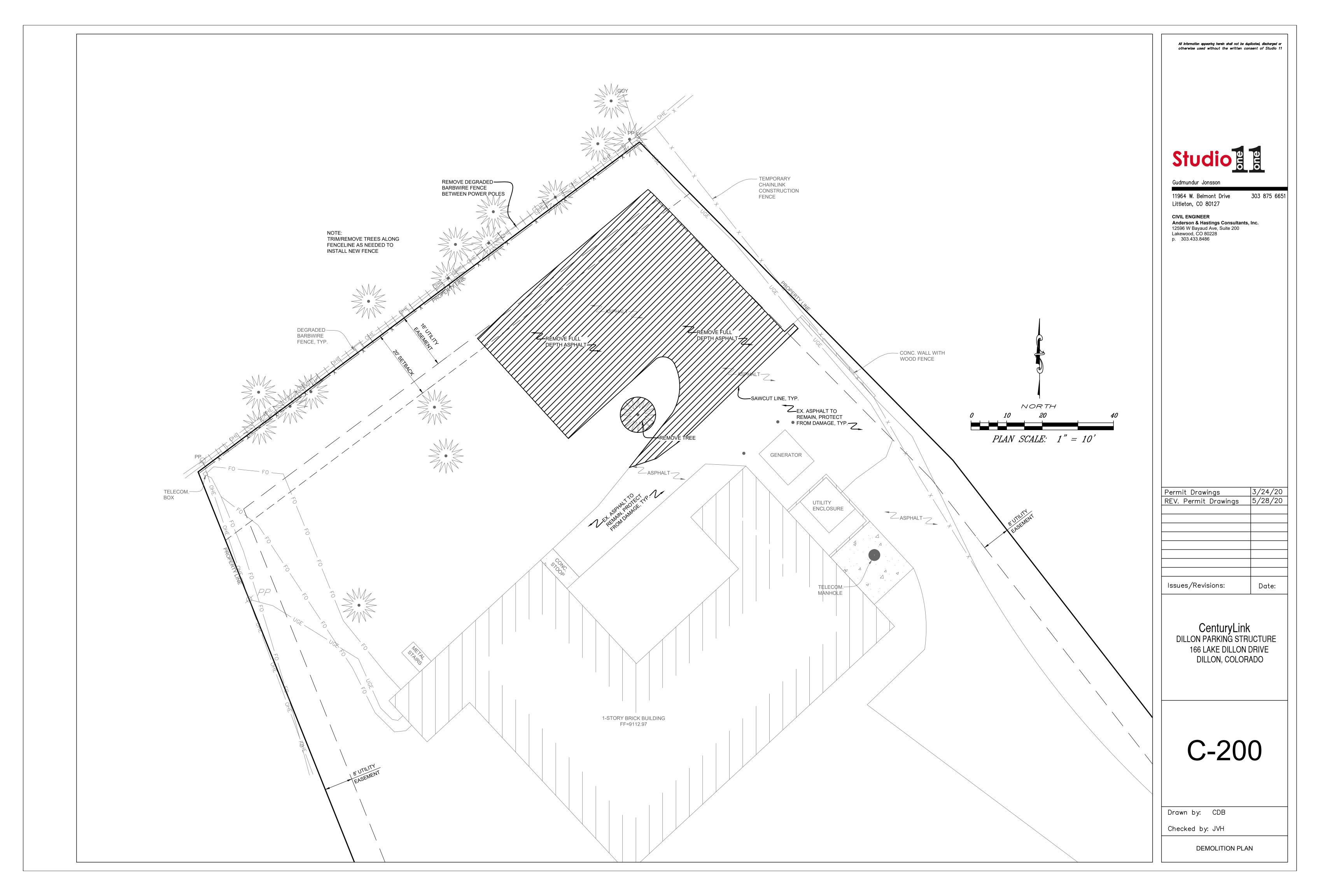
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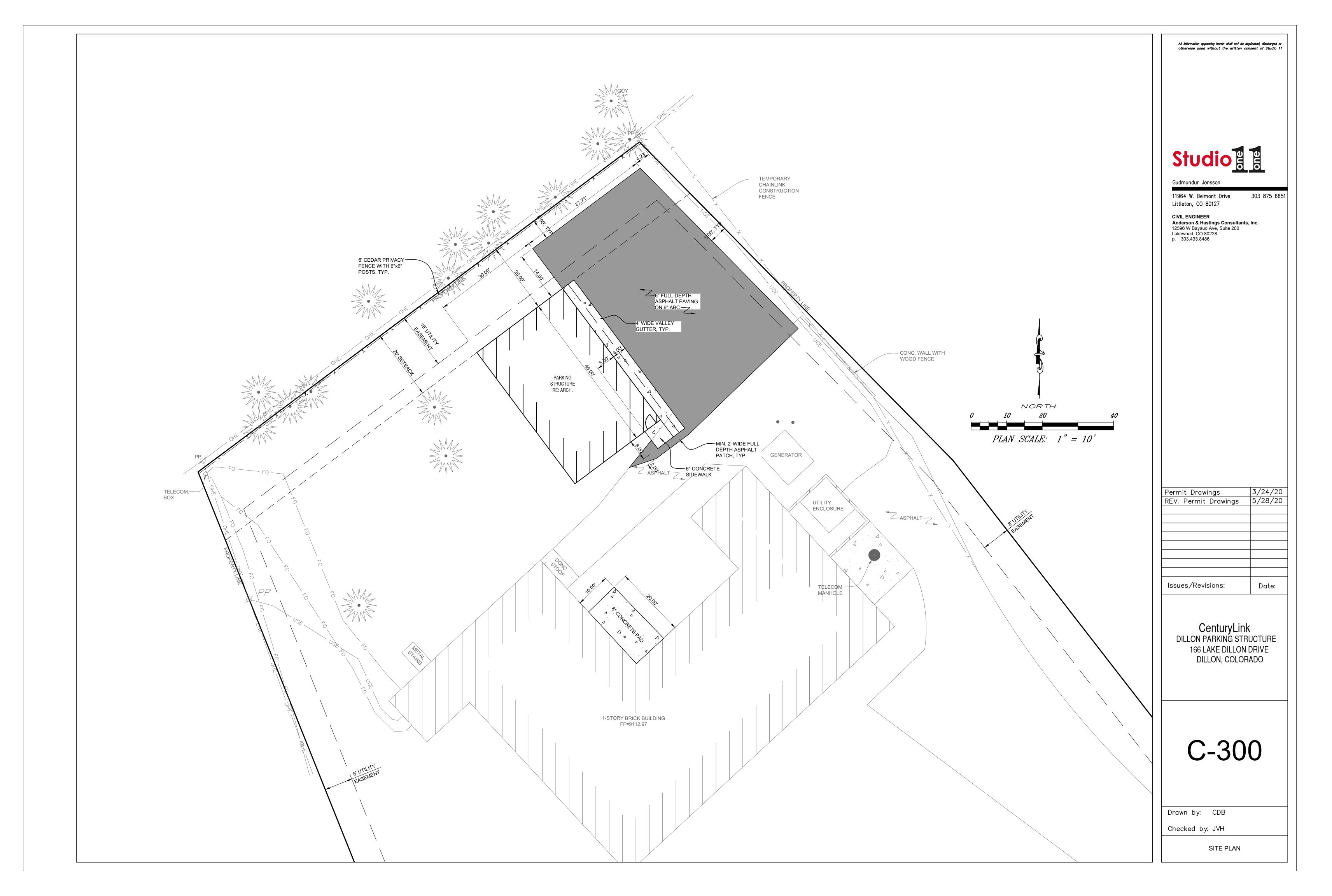
> CenturyLink DILLON PARKING STRUCTURE 166 LAKE DILLON DRIVE DILLON, COLORADO

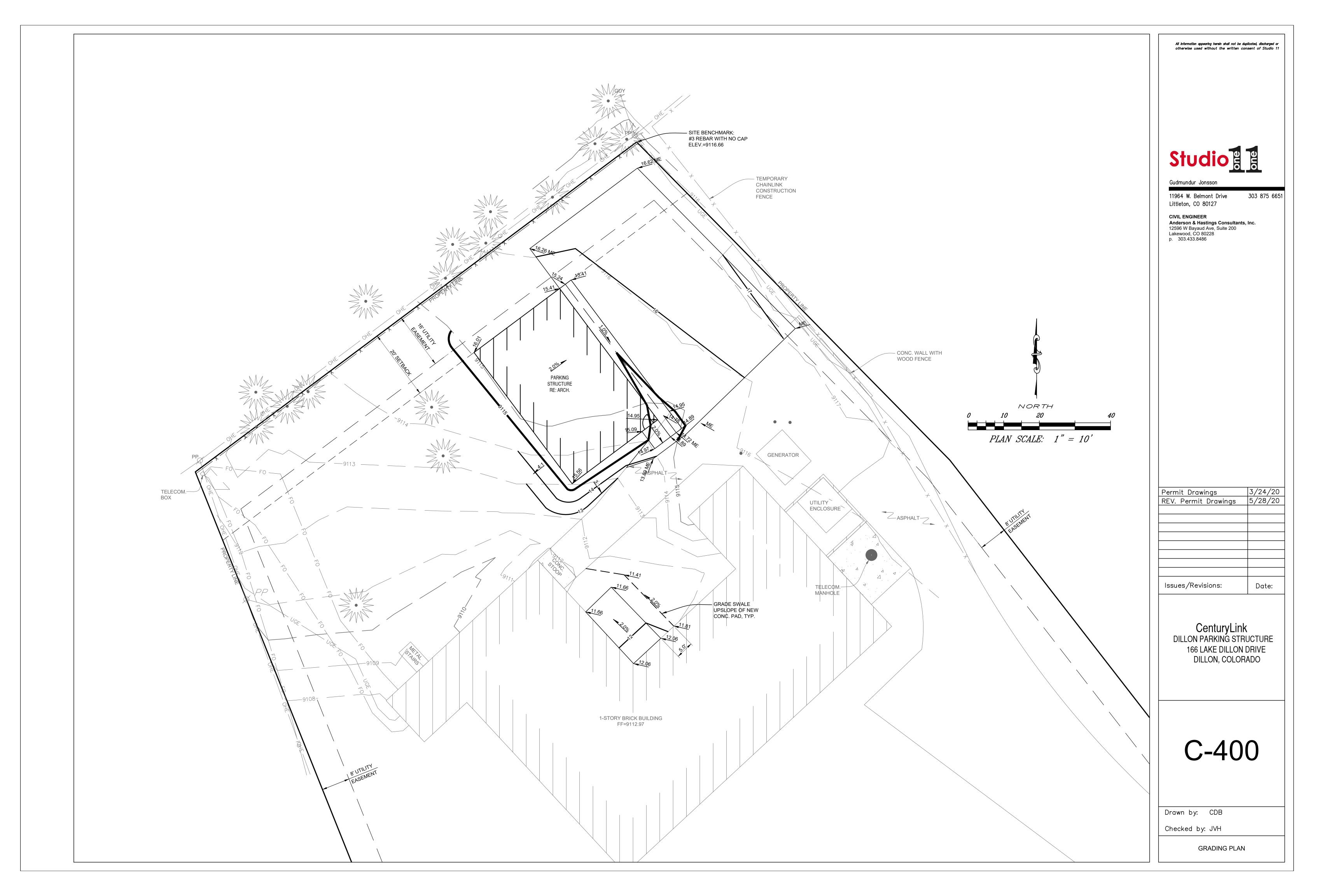
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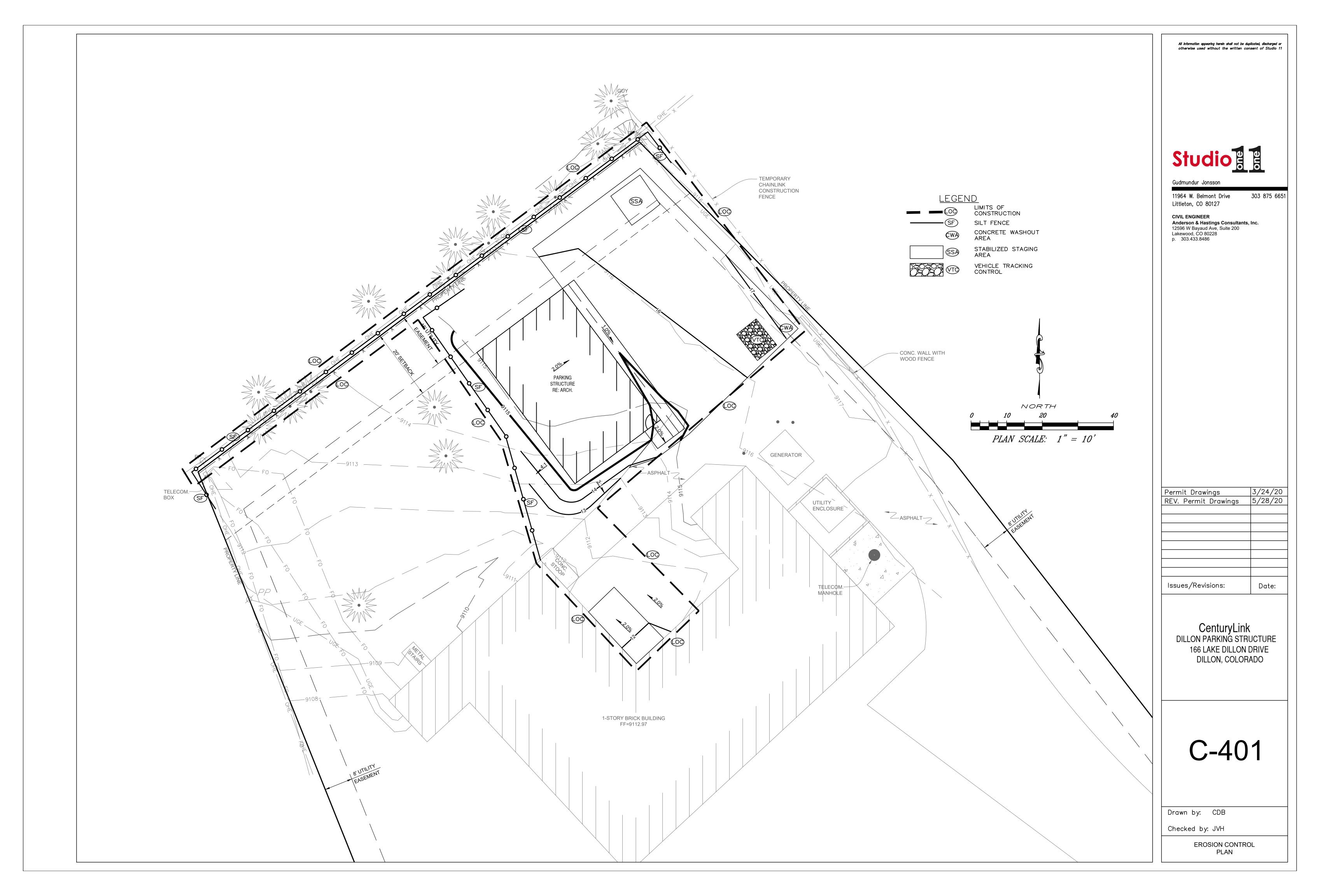
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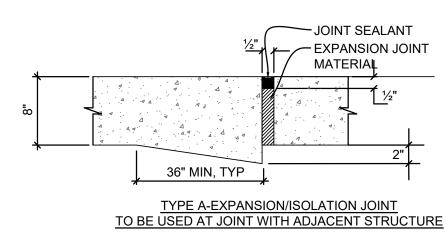
CIVIL **SPECIFICATIONS**

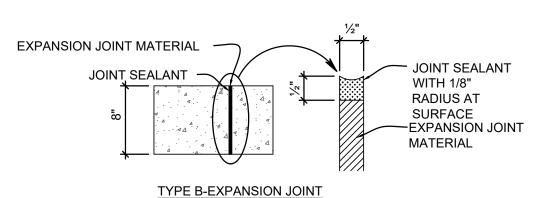




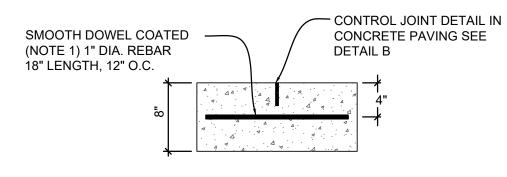






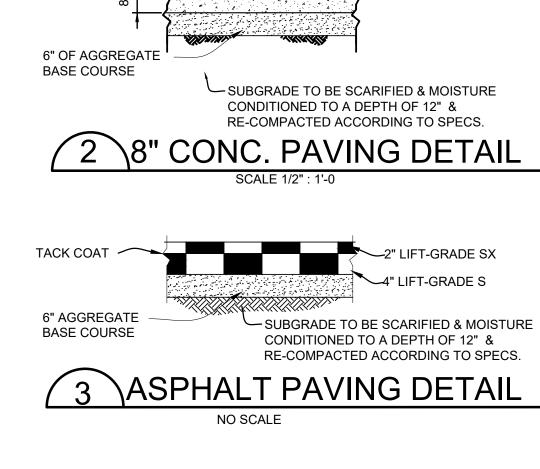


IN CONCRETE PAVING



CONCRETE PAVING NOTES:

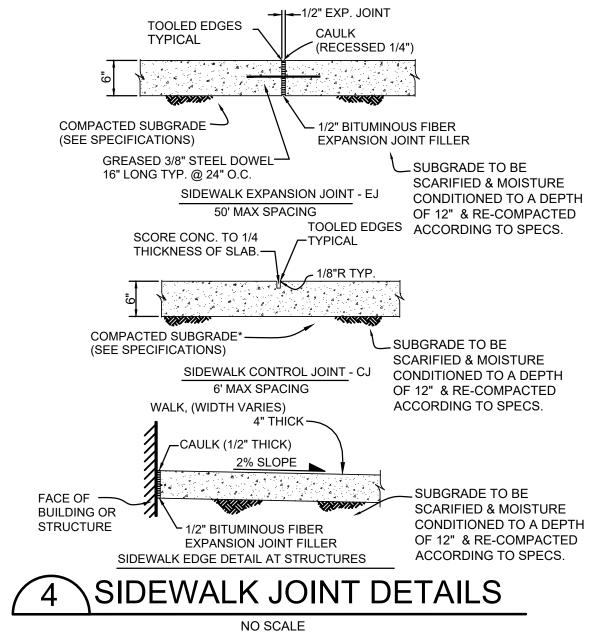
- 1. REINFORCEMENT: ASTM A 615, GRADE 60, EPOXY COATED DEFORMED STEEL REBAR OR SMOOTH STEEL DOWELS WITH DIAMETER AND LENGTH AS INDICATED.
 - A. SPACE DOWELS AT 12 ON CENTER.
 - B. GREASE DOWELS TO PROVIDE MOVEMENT IN EXPANSION JOINTS.
 - C. SPACE TIE BARS AT 30" ON CENTER D. KEEP TIE BARS IN THE VERTICAL CENTER OF THE CONCRETE SLAB AND PERPENDICULAR TO THE JOINT DURING CONCRETE PLACEMENT.
- 2. SAW CONTROL JOINTS (CONTRACTION JOINTS) BEFORE SHRINKAGE CRACKING TAKES PLACE. DO NOT TEAR OR RAVEL CONCRETE DURING SAWING. IN COOL WEATHER, THE JOINT SAWING MAY BE DELAYED ONLY FOR THE TIME REQUIRED TO PREVENT TEARING AND RAVELING THE CONCRETE. CUT CONTROL JOINTS TO DIMENSIONS RECOMMENDED BY SEALANT MANUFACTURER AND APPROVED BY CONTRACTING OFFICER.
- 3. JOINTS: LAY OUT JOINTS TO AID CONSTRUCTION AND CONTROL RANDOM CRACKING.
 - A. CONTROL JOINT SPACING SHALL BE 15 FEET MAXIMUM ON CENTER IN BOTH
 - B. EXTEND TRANSVERSE CONTROL JOINTS CONTINUOUSLY ACROSS THE FULL WIDTH OF
 - THE CONCRETE. C. MAKE ADJUSTMENTS IN JOINT LOCATIONS TO MEET INLET OR MANHOLE LOCATIONS. D. EXPANSION JOINTS SHALL BE PLACED WHERE CONCRETE ABUTS A BUILDING WALL, SIDEWALK, CURB, GUTTER OR ANY IMMOVABLE STRUCTURE.
- 4. EXPANSION JOINT MATERIAL:
- BITUMINOUS (ASPHALT OR TAR) MASTIC, ASTM D994. FORMED AND ENCASED BETWEEN 2 LAYERS OF BITUMINOUS SATURATED FELT OR 2 LAYERS OF GLASS-FIBER FELT EXTENDING TO THE BOTTOM OF THE CONCRETE SLAB.
- 5. BACKER ROD:
- ROUND RODS. RODS MUST BE OVERSIZED APPROXIMATELY 25 PERCENT TO FIT TIGHTLY INTO EACH JOINT AND COMPATIBLE WITH HOT POURED SEALANT.
- SILICONE ASTM D5893, ON CDOT LIST OF APPROVED MATERIALS.
- 7. JOINT NOT REQUIRED BETWEEN PAVEMENT AND CURB & GUTTER IF POURED

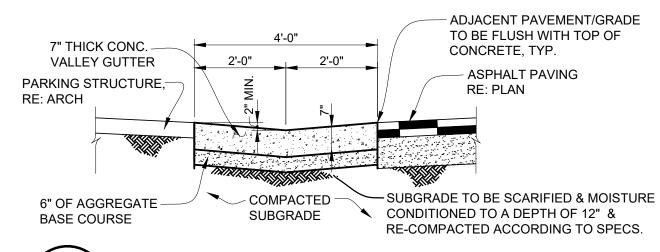


- 8" CONC. PAVING

SHEET

RE: DETAIL 1, THIS





VALLEY GUTTER DETAILS

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Issues/Revisions:	Date:

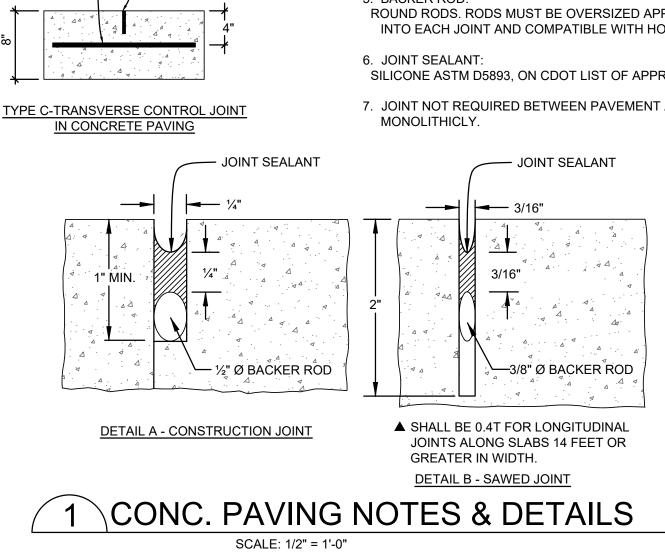
CenturyLink
DILLON PARKING STRUCTURE 166 LAKE DILLON DRIVE DILLON, COLORADO

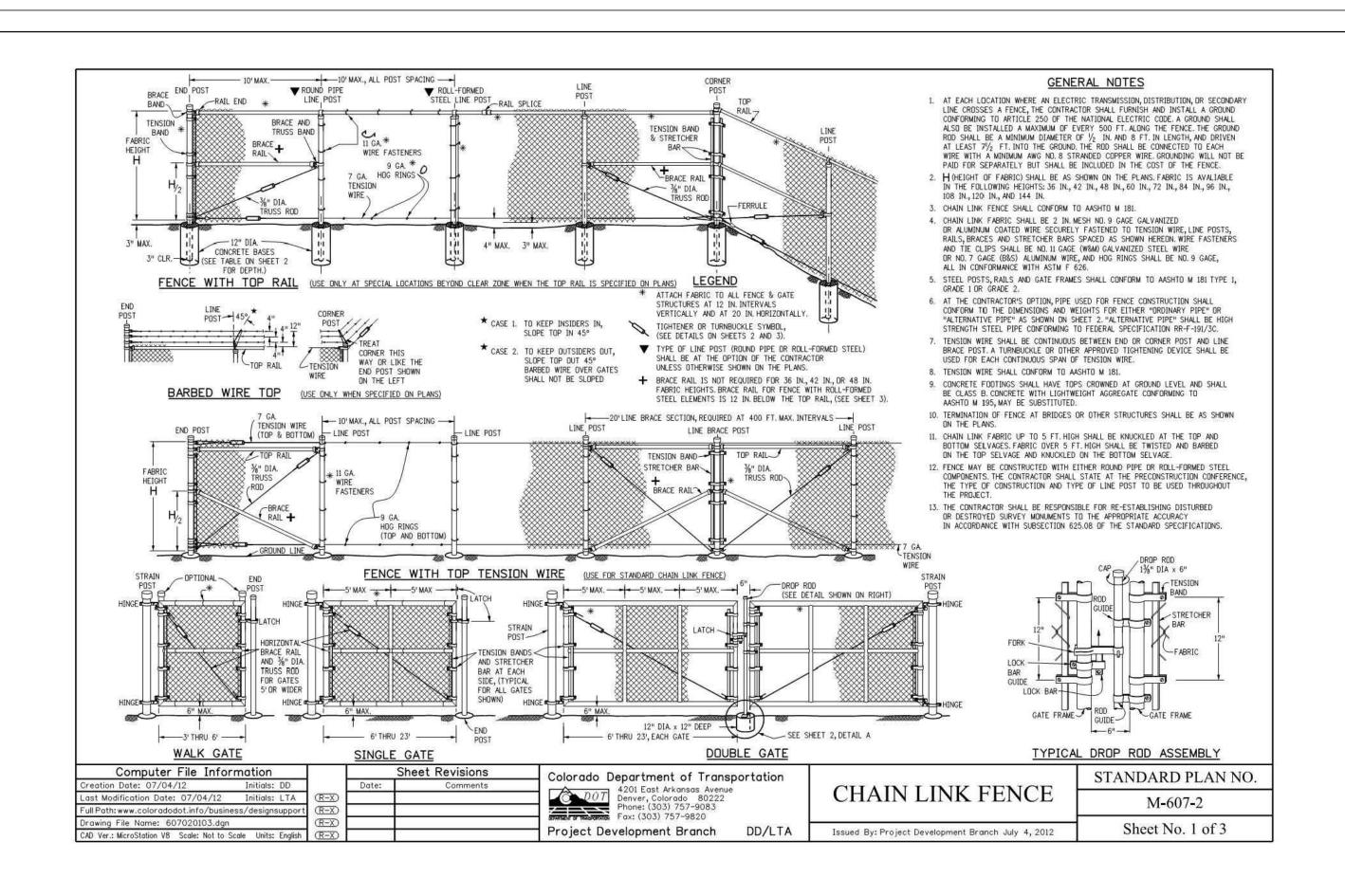
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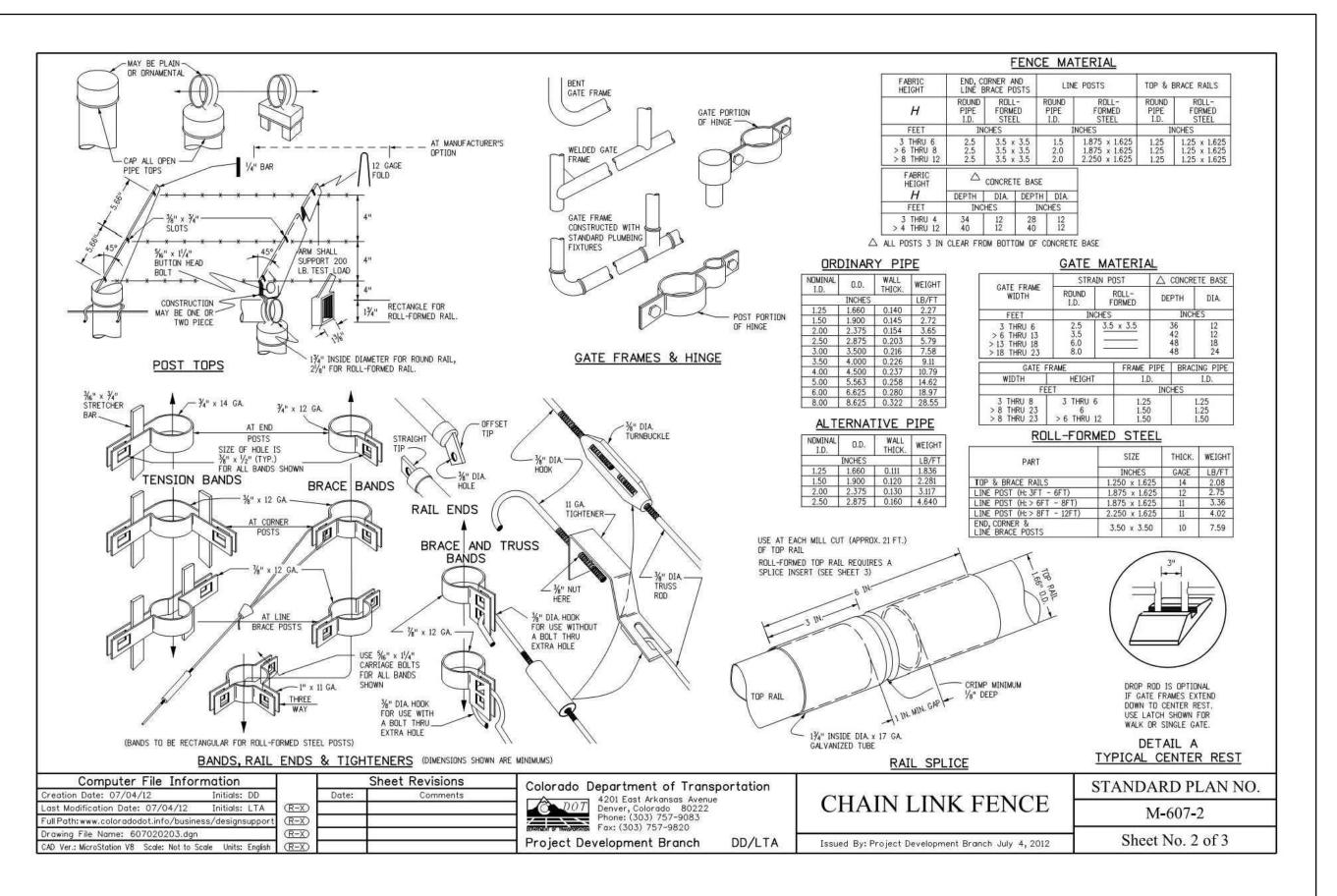
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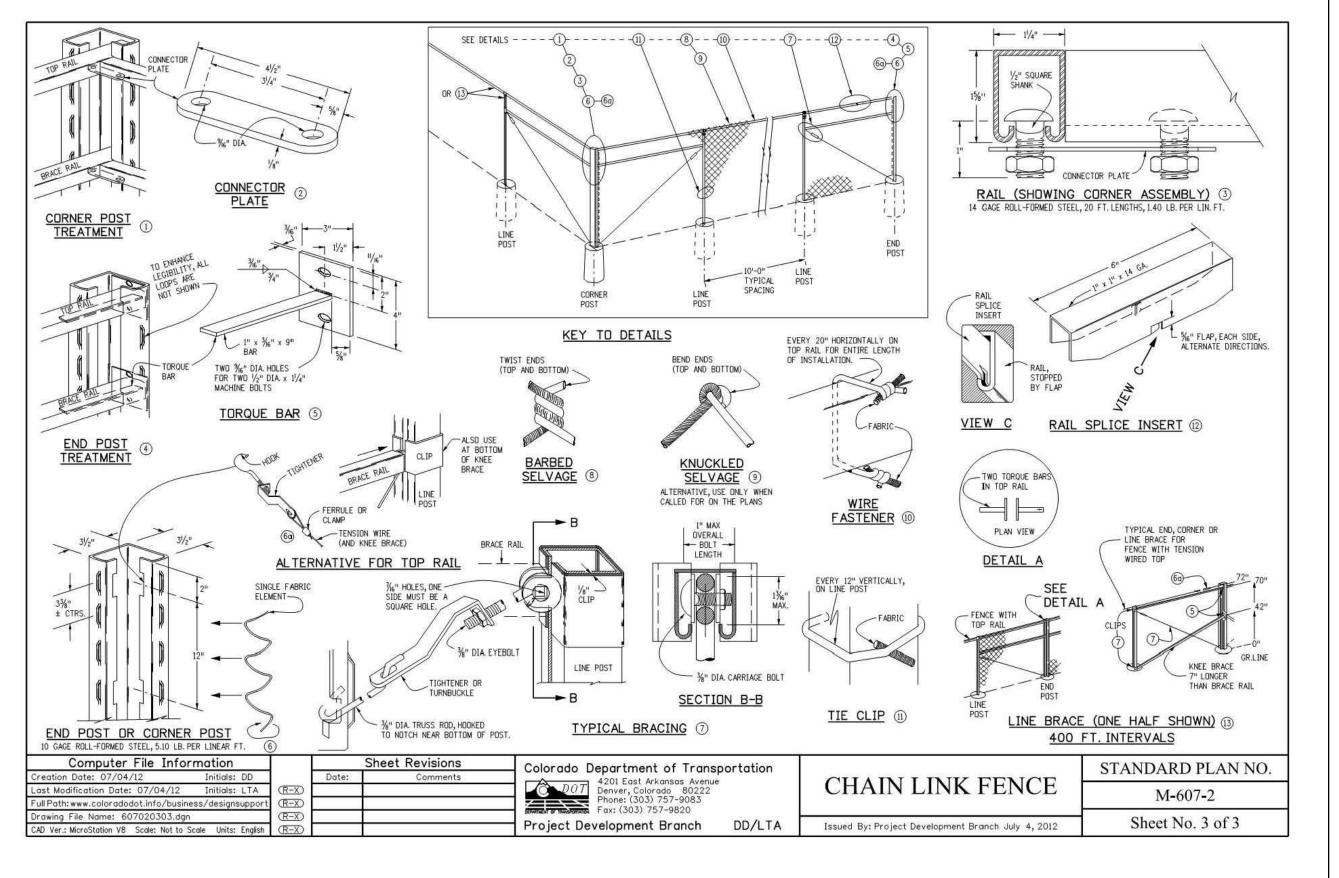
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SITE DETAILS









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	Issues/Revisions:	Date:

CenturyLink
DILLON PARKING STRUCTURE 166 LAKE DILLON DRIVE DILLON, COLORADO

C-501

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November 2010

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Checked by: JVH

EROSION CONTROL DETAILS

MORTON BUILDINGS GENERAL SPECIFICATIONS

LAMINATED COLUMNS - NO. 1 OR BETTER SOUTHERN YELLOW PINE NAIL LAMINATED 3 MEMBER S4S COLUMNS NAILED 8" O.C. STAGGERED ON EACH SIDE WITH 4" NAILS.

MFS PRE-CAST CONCRETE COLUMN - MORTON BUILDINGS FOUNDATION SYSTEM IS A PRE-ENGINEERED, 10,000 PSI, STEEL REINFORCED COLUMN FOR BELOW GROUND INSTALLATION. DESIGNED TO BE MECHANICALLY FASTENED TO ABOVE GROUND NAIL LAMINATED COLUMNS. THE SYSTEM IS DESIGNED TO RESIST BOTH AXIAL AND BENDING FORCES.

FOOTINGS AND ANCHORAGE - COLUMN HOLES ARE DUG A MINIMUM DEPTH OF 4'-0" BELOW GRADE (SEE PLANS FOR DIAMETER AND DEPTH). MFS PRE-CAST CONCRETE COLUMNS ARE PLACED IN THE HOLE. CONCRETE (MINIMUM COMPRESSIVE STRENGTH 2500 PSI) IS POURED IN PLACE TO THE SPECIFIED THICKNESS (SEE PLANS FOR REQUIRED THICKNESS ABOVE AND BELOW THE COLUMN). THE COLUMN IS THEN BACKFILLED WITH SOIL AND COMPACTED AT 8" INTERVALS OR BACKFILLED WITH CONCRETE (SEE PLANS).

TREATED LUMBER -- PRESSURE PRESERVATIVE TREATED LUMBER OTHER THAN LAMINATED COLUMNS ARE NO. 1 OR BETTER SOUTHERN YELLOW PINE AND CENTER MATCHED OR NOTCHED AND GROOVED OR \$4\$. PRESSURE TREATMENT TO GROUND CONTACT RETENTION WITH PRESERVATIVE TREATMENT COMPLYING WITH USE CATEGORY UC4B (AWPA OR ICC-ES) AND IN COMPLIANCE WITH USEPA GUIDELINES AND STANDARDS.

FRAMING LUMBER - SIDING NAILERS ARE 2x4 S4S OR 2x6 SPF NO. 2 OR BETTER SPACED APPROXIMATELY 36" O.C. WITH ALL JOINTS STAGGERED AT ATTACHMENT TO COLUMNS. ROOF PURLINS ARE 2x4 \$4\$ NO. 2 OR BETTER ON EDGE SPACED APPROXIMATELY 24" O.C. ALL OTHER FRAMING LUMBER IS NO. 2 OR BETTER.

ROOF TRUSSES - FACTORY ASSEMBLED WITH 18 OR 20 GAUGE GALVANIZED STEEL TRUSS PLATES AS REQUIRED AND KILN DRIED LUMBER AS SPECIFIED, IN-PLANT QUALITY CONTROL INSPECTION IS CONDUCTED UNDER THE AUSPICES OF THE TPI INSPECTION BUREAU. TRUSSES ARE DESIGNED IN ACCORDANCE WITH CURRENT STANDARDS AND SPECIFICATIONS FOR THE STATED LOADING.

SIDING & ROOFING PANELS (FLUOROFLEX 1000 ™) - 0.019" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL WITH AN ADDITIONAL BAKED-ON 70% PVDF FINISH WITH A NOMINAL 1 MIL. PAINT THICKNESS ON EXTERIOR.

TRIM - DIE-FORMED TRIM OF 0.017" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL ON GABLES, RIDGES, CORNERS, BASE WINDOWS, AND DOORS WITH SAME FINISH AS ROOFING OR SIDING PANELS.

GUTTERS - 5" K-STYLE, .030 HIGH TENSILE ALUMINUM GUTTER, 70% PVDF FINISH TO MATCH TRIM, ON BOTH SIDES OF THE BUILDING. 2x4F1F1 02/12

	SHEET INDEX		
SHEET#	DESCRIPTION		
G1 OF G1	SPECIFICATIONS & SHEET INDEX		
\$1 OF \$6	COLUMN PLAN		
S2 OF S6	TRUSS PLAN, TRUSS DRAWING, & DETAILS		
\$3 OF \$6	ELEVATIONS		
S4 OF S6	SECTIONS & DETAILS		
\$5 OF \$6	SECTIONS & DETAILS		
S6 OF S6	SECTIONS & DETAILS		

CURRE	CURRENT LUMBER SPECIFICATIONS (06-01-2013)		
SIZE	DESCRIPTION	BENDING VALUE Fb	
2x4	NO. 2 SPF	1313 PSI	
2x4	NO. 1 SYP	1500 PSI	
2x4	2100f MSR SPF	2100 PSI	
2x6	NO. 2 SPF	1138 PSI	
2x6	NO. 1 SYP	1350 PSI	
2x6	2100f MSR SPF	2100 PSI	
2X6	2400 MSR SYP	2400 PSI	
2x8	NO. 1 SYP	1250 PSI	
2x8	2400 MSR SYP	2400 PSI	
2x10	NO. 1 SYP	1050 PSI	
2x10	2400 MSR SYP	2400 PSI	
2x12	NO. 1 SYP	1000 PSI	
2x12	2250f MSR SYP	2250 PSI	
1 1/2"x16"	LAMINATED VENEER LUMBER	2800 PSI	
3 1/2"x15"	GLU-LAM	1650 PSI	
5 1/4"x16 1/2"	GLU-LAM	2400 PSI	
5 1/4"x19 1/2"	GLU-LAM	2400 PSI	

DESIGN AND EXPLANATORY NOTES

- 1.) ALL PLOT PLANS AND RELATED DETAILS SHALL BE PROVIDED BY OWNER UNLESS INCORPORATED AS PART OF THESE DRAWINGS.
- 2.) MORTON BUILDINGS GENERAL SPECIFICATIONS APPLY UNLESS INDICATED DIFFERENTLY ON SPECIFIC JOB DRAWINGS OR SUPPLEMENTAL INFORMATION.
- 3.) MINIMUM LIVE ROOF LOAD DESIGNS FOR CONSTRUCTION, MAINTENANCE, REPAIR, AND OTHER TEMPORARY LOADS PER SECTION 1607.12.2
- a.) ROOF PURLINS AND OTHER SECONDARY STRUCTURAL MEMBERS = 20 PSF
- b.) ROOF TRUSSES, HEADERS, COLUMNS AND OTHER PRIMARY STRUCTURAL MEMBER = 18 PSF
- c.) FOOTINGS = 12 PSF (DESIGNED FOR ROOF SNOW LOAD AND OTHER NON-TEMPORARY LOADS W/ APPROVAL FROM BUILDING OFFICIAL).
- 4.) NO ONE MAY ALTER ANY ENGINEERING ITEM UNLESS ACTING UNDER THE DIRECTION OF THE LICENSED / REGISTERED ENGINEER.
- 5.) ◆ THE PRECEDING SYMBOL IDENTIFIES ITEMS THROUGHOUT THE PLANS THAT ARE NOT PROVIDED BY MORTON BUILDINGS, INC. OR MORTON BUILDINGS' SUBCONTRACTORS AND ARE THE OWNER'S RESPONSIBILITY.

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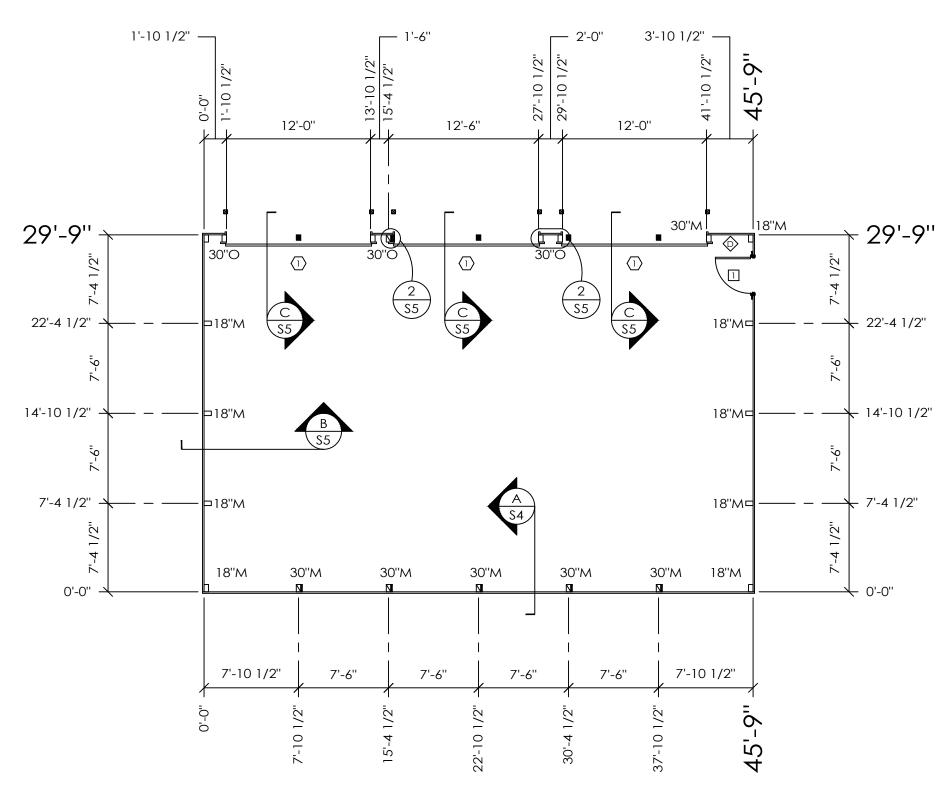
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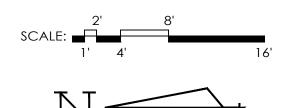
2012 IBC AND SUMMIT COUNTY BUILDING CODE AMENDMENTS

BUILDING DESIGN CRITERIA		
USE GROUP	S-1	
CONSTRUCTION TYPE	VB	
RISK CATEGORY		
BUILDING AREA	1380 SQ. FT.	
ROOF SNOW LOAD *	70 PSF	
GROUND SNOW LOAD	70 PSF	
WIND SPEED (VASD)	90 MPH	

ROOF SNOW LOAD = GROUND SNOW LOAD PER SUMMIT COUNTY BUILDING CODE

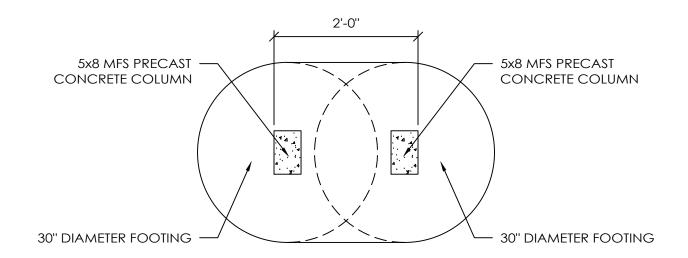


COLUMN PLAN



COLUMN PLAN LEGEND

- 3-2x8 LAMINATED COLUMN LOCATION
- N 3-2x8 LAMINATED COLUMN LOCATION W/ ADDITIONAL 2x8 LAMIANTE
- HEADERED TRUSS LOCATION
- 3068 MB910 PLAIN FLAT LEAF WALKDOOR, IN SWING, LEFT HINGE WITH KEYPAD ENTRY (RESIDENTIAL) & CLOSER
- (3) 12'-2" x 14'-0" OVERHEAD DOORS W/ 4" x 4" JAMB PROTECTORS
 - BUILDING SEAL PACKAGE
 - ALL EXTERIOR STEEL FASTENED W/ STAINLESS STEEL SCREWS
- ♦ DOUBLE LAYER 3/4" OSB SHEARWALL LOCATION (SEE DETAILS ON SHEET S6)
- 18"M 18" DIAMETER FOOTING WITH 4' EMBEDMENT TO BOTTOM OF 21" THICK CONCRETE PAD (2500 PSI MINIMUM). 20" BELOW BOTTOM OF PRECAST CONCRETE COLUMN AROUND EXPOSED REBAR CAGE AND 3/4"x14" THREADED ROD WITH AN ADDITIONAL MINIMUM 1" ABOVE BOTTOM OF PRECAST CONCRETE COLUMN. PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION.
- 30"M 30" DIAMETER FOOTING WITH 4' EMBEDMENT TO BOTTOM OF 21" THICK CONCRETE PAD (2500 PSI MINIMUM). 20" BELOW BOTTOM OF PRECAST CONCRETE COLUMN AROUND EXPOSED REBAR CAGE AND 3/4"x14" THREADED ROD WITH AN ADDITIONAL MINIMUM 1" ABOVE BOTTOM OF PRECAST CONCRETE COLUMN. PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION.
- 30"O COMBINED 30" DIAMETER OVAL FOOTING WITH 4' EMBEDMENT (SEE DETAIL #1 BELOW).



COMBINED FOOTING DETAIL #1

SCALE: 3/4" = 1'-0"

(TYPICAL DETAIL - APPLIES TO VARIOUS COLUMN SPACINGS - SEE COLUMN PLAN ABOVE)

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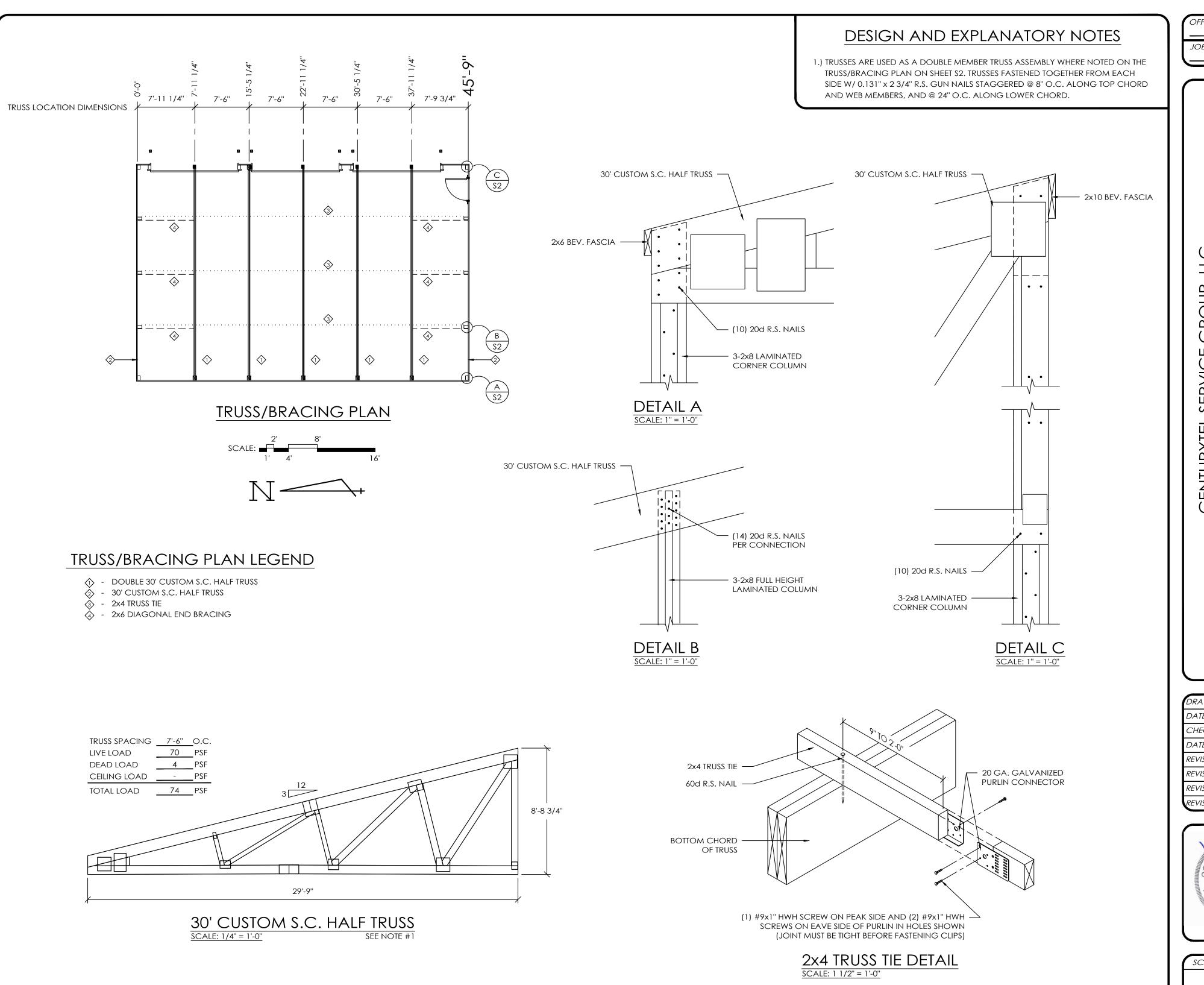
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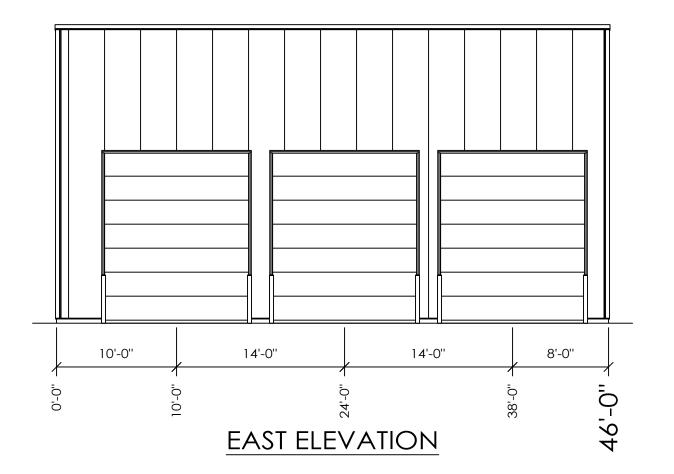
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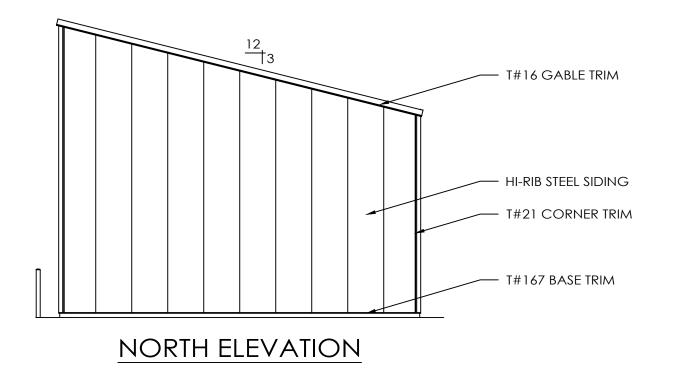
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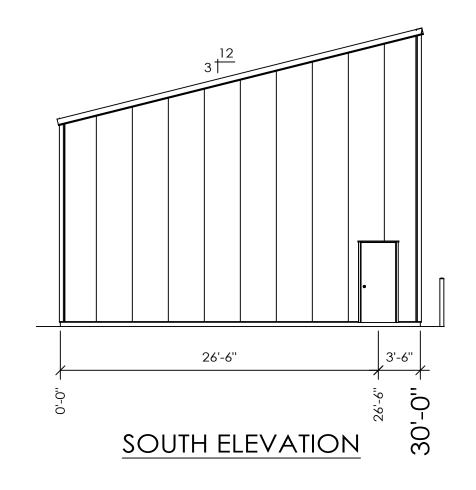
DESIGN AND EXPLANATORY NOTES

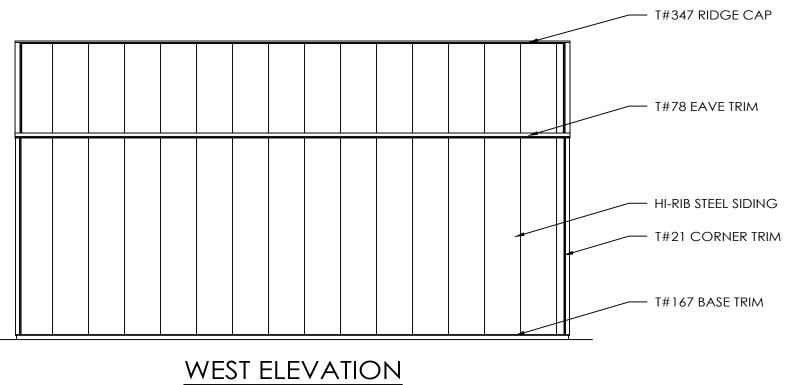


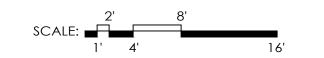
1.) EXTERIOR DOOR LOCATIONS ARE TAKEN FROM THE EXTERIOR FACE OF THE NAILERS AND ARE TO THE CENTER OF THE DOOR UNITS. VERIFY ALL DOOR LOCATIONS WITH THE OWNER.











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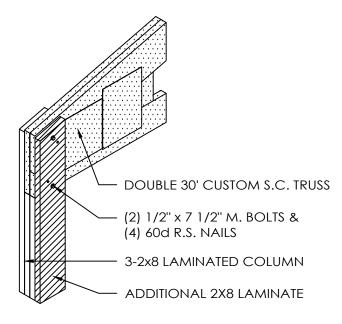
FLUOROFLEX 1000 HI-RIB STEEL 2x6 (2100 MSR SPF) PURLINS @ 20" O.C. W/ SIMPSON PFDS26B HANGERS FILLER STRIPS T#78 EAVE TRIM DOUBLE 30' CUSTOM S.C. HALF TRUSS 2x6 BEV. FASCIA -(2) 1/2" x 7 1/2" M. BOLTS & (4) 20d R.S. NAILS 3-2x8 LAMINATED COLUMN W/ ADDITIONAL 2x8 LAMINATE TO TOP OF MFS - FLUOROFLEX 1000 HI-RIB STEEL (5) ROWS OF 2x4 NAILERS (2100 MSR SPF) 16'-0'' 2x2 VERTICAL BLOCKING **GRADE TO HEEL** AT COLUMN LOCATION (32) 1/4" x 2 1/2" POWER LAG WASHER HEAD YELLOW ZINC SCREWS FILLER STRIP — T#167 BASE TRIM 2x8 TREATED SPLASHBOARD (W/ 360M & 370M BRACKETS) 6" CONCRETE FLOOR FINISH 4" MINIMUM COMPACTED GRANULAR GRADE BASE OR IN SITU GRANULAR SOIL ◆ 5x8 MFS PRECAST CONCRETE COLUMN 4'-0'' 21" THICK CONCRETE PAD (2500 PSI MINIMUM). 20" BELOW BOTTOM OF PRE-CAST CONCRETE COLUMN AROUND EXPOSED REBAR CAGE AND 3/4" x 14" THREADED ROD WITH AN ADDITIONAL MINIMUM 1" ABOVE BOTTOM OF PRE-CAST CONCRETE COLUMN. PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION. 30''Ø

SIDEWALL SECTION A

SCALE: 1/2" = 1'-0"

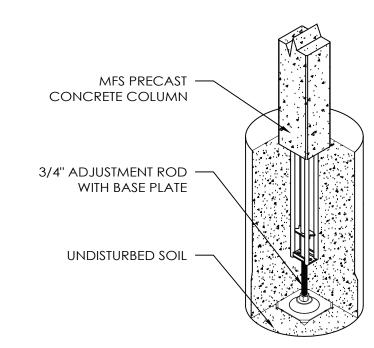
DESIGN AND EXPLANATORY NOTES

- 1. FOOTINGS ARE DESIGNED FOR A 2000 PSF SOIL BEARING CAPACITY. LOCAL CONDITIONS MAY REQUIRE MODIFICATIONS.
- 2. CONCRETE FLOOR NOTES:
 - a. 3500 PSI, 5 1/2 BAG MIX CONCRETE.
 - b. SLOPE GRADE AWAY FROM BUILDING @ 1" PER FOOT FOR A MINIMUM DISTANCE OF 10' PLUS OVERHANG WIDTH.
 - C. A VAPOR RETARDER IS NOT MANDATED PER IBC SECTION 1907 EXCEPTION 3. UNLESS THE FLOOR WILL BE COVERED BY MOISTURE SENSITIVE FLOORING MATERIALS OR IMPERMEABLE FLOOR COATINGS OR WHERE THE FLOOR WILL BE IN CONTACT WITH ANY MOISTURE SENSITIVE EQUIPMENT OR PRODUCT.
 - d. CONTRACTION JOINTS UNIFORMLY SPACED 18' O.C. OR LESS.
- 3. PRIOR TO PLACING THE CONCRETE FOOTINGS, HAND TAMP THE BOTTOM 2"-3" OF LOOSE SOIL TO CONSOLIDATE. IF THE DRILLED HOLE CONTAINS MORE THAN 3" OF LOOSE SOIL, REMOVE EXCESS SOIL TO A UNIFORM THICKNESS OF 2"-3", HAND TAMP AND PROCEED WITH CONCRETE FOOTING PLACEMENT.
- 4. DO NOT PLACE CONCRETE FOOTING THROUGH MORE THAN 3" OF STANDING WATER. IF MORE THAN 3" OF STANDING WATER IS PRESENT IN THE FOOTING HOLE CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR INSTALLATION INSTRUCTIONS.



TRUSS/COLUMN CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



LOWER COLUMN INSTALLATION

- 1. INSTALL PRECAST CONCRETE COLUMN W/ADJUSTMENT ROD & BASE PLATE IN THE AUGERED HOLE.
- 2. PLUMB PRECAST CONCRETE COLUMN IN BOTH DIRECTIONS
- 3. ADJUST HEIGHT UP OR DOWN WITH ADJUSTMENT HEX ROD
- 4. POUR READI-MIX CONCRETE INTO THE HOLE AS SPECIFIED.
- 5. BACKFILL AND COMPACT THE ANNULAR SPACE AROUND THE COLUMN TO GRADE WITH SOIL AUGERED FROM THE SITE.

LOWER COLUMN ISOMETRIC DETAIL

SCALE: 3/4" = 1'-0"

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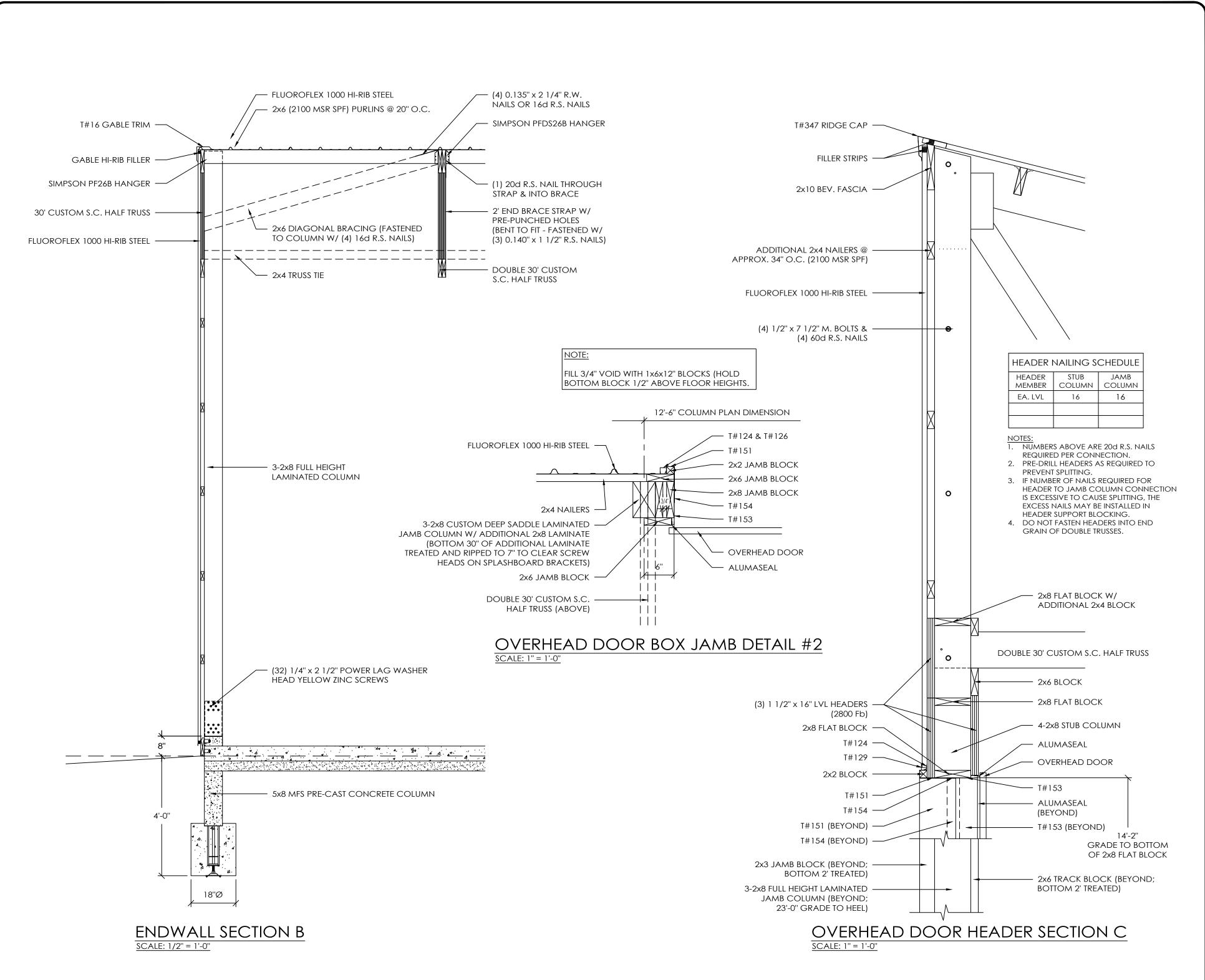
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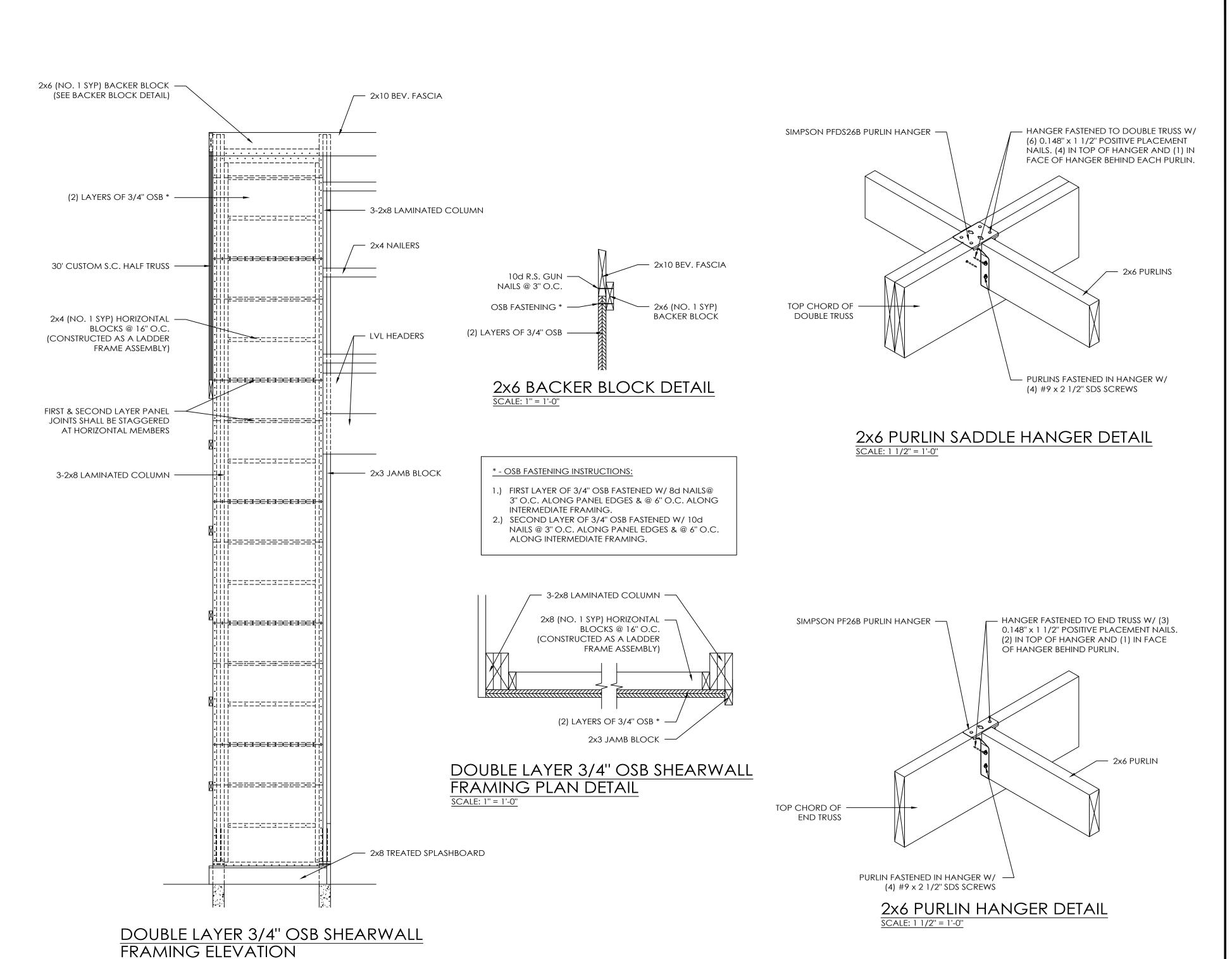
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\$5 of \$6



SCALE: 1/2" = 1'-0"

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145-097202

CENTURYTEL SERVICE GROUP,

DILLON, (

DRAWN BY:	BISHOP
DATE:	4/13/2020
CHECKED BY:	CES
DATE:	4/16/2020
REVISED DATE:	
KEVISED DATE:	



SCALE: AS NOTED

SHEET NO.

S6 OF S6